



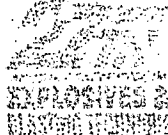
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## 서울 지하철 건설의 발파기술 발전

ON THE DEVELOPMENT OF EXPLOSION TECHNOLOGY  
IN SEOUL METRO SUBWAY CONSTRUCTION

\* 故 許 墳 會長 Ginn Huh

### ABSTRACT

The blasting work to construct a subway in Seoul, Korea have often caused increased neighbor's complaints because of ground vibration.

In order to prevent the damage to the structure, it was necessary to predict the level of blasting induced vibration and to determine the maximum charge weight per delay within an allowable vibration level.

The effect of blasting pattern, rock strength and different explosives on the blast-induced ground vibration was studied to determine the maximum charge weight per delay within a given vibration level. The blasting vibration equation from over 100 test data was obtained,  $V=K(D/W^{1/3})^n$ , where the values for n and K are estimated to be 1.7 to 1.5 and 48 to 138 respectively. See Table 1

\* 大韓火藥技術學會 前任 會長  
APEC 審査登錄委員會 韓國代表  
東南亞太平洋工學會聯合(FEISEAP) 副會長  
(2000年 3月부터 後任 會長으로 選出됨)

Sequential blasting machine was applied to the site.

Tunneling have been working so carefully in order to decrease vibration and Noise to near shopp and housing area.

We carried out empirical formula to solve city environmental pollution as follow;

#### Empirical formula

$$\text{For Granite : } V = KW^{0.57}D^{-1.75}$$

$$\text{For Gneiss : } V = KW^{0.5}D^{-0.5}$$

$$\text{For Concrete breaker : } V = KW^{0.5}D^{-1.75} = 7 \times 0.06^{0.5}D^{-1.75}$$

W = Amount of Power/delay kg

D = Distance m

V = Partical vibration velocity cm/sec

K = Coefficiency= $E_i (R_i, S_c + Q_i)$

$S_c$  : Compressive St. kg/cm<sup>2</sup>

$E_i$  : Power Compensation Ratio  $\left\{ \begin{array}{l} \text{Dynamite} = 1 \\ \text{Slurry} = 0.8 \\ \text{AN} = 0.65 \end{array} \right.$

R : Rock Coefficiency

Seoul Granite = 0.0371

Seoul Gneiss = 0.0206

Q : Compensation by blasting pattern

Apply distance ; -30m

Bit Gage ;  $\Phi$  38mm

Explosive ; Emulsion

Electric caps ; M/S

Pattern ; Bench cut Tunnel

## Subways in Seoul

Seoul, with more than 10 million people, is one of the most populated cities in the world. The 3rd and 4th subway line in Seoul cross each other and run north-south direction, passing through the metropolitan areas. After completing the construction, Seoul has 120.6km subway lines in total length and capable of transporting 5 million passengers per day at 3 to 7 minute intervals.

It is also estimated that about forty percent of the total traffic population of Seoul is moved by subway lines, thirty-two by buses and twenty-eight by other means of public transportation.

## Physical Properties of rock

The geology of the construction areas is mainly composed of Precambrian gneiss as the base rock intruded by jurassic granite and overlain by alluvium as an unconformity. The quality of base rock and thickness of alluvium are severely changed from location to locations.

The detailed geologic conditions were surveyed to know the rock quality, the direction and openness of joint, the extent of ground water inflow and the bedding planes of the strata, etc.

The velocity and the frequencies of a vibration generated by a detonation in rock mass depend on the type of used explosives, the type of blasting and especially on the elastic, physical and mechanical properties of the rock that transmits the vibration.

The detailed of rock properties are given in Table 2.

Rock samples were obtaining from in-situ rock and prepare as cylindrical cores of 42mm inner diameter.

The relations between Schmidt rebound hardness, P-wave velocity and uniaxial compressive strength could be represented by the following equations.

$$Sc = 0.0514 \times (S.H)^{2.3} : (1)$$

where sc : Uniaxial compressive strength (MPa)

S.H : Schmidt rebound hardness

Fig. 1 and Fig. 2 are shown the relations.

If the value of Schmidt rebound hardness for in-situ rock is known, the compressive strength could be

approximately estimated.

### Empirical Formula of standard blasting test

In case of tunnel blasting, there is only free face the tunnel heading. After the center holes were blasted, the works, which remain, is the implementation of bench cut against the opening to make the full sectional area required. The quantity of explosives to be charged, however, is hardly estimated, as rocks very seldom show any sign of homogeneous quality.

Experimental tests therefore have been implemented to calculate the specific charge of the explosives of the certain strength, the spacing of holes and the diameter of holes to be drilled.

A series of holes are drilled at 800mm behind the face to a depth of 1,200mm and firings are implemented at each hole with varied charge of explosives until the burden is teared off. Should it be realized, the specific charge of the rock to be blasted can be calculated by the following formula

$$Ca = \frac{A}{SW} \quad \text{where as } A = m \text{ activated area}$$

S = Peripheral length of Charged room

Ca = Rock Coefficient

Di = Holes diameter

Later in 1980, the Dynamite Explosive was replaced into Emulsion & Milli-Second Delay Electric Cap.

Table 4 Standardization in tunneling

Fig. 3 Tunnel Blasting Drilling Pattern

### Production of ground vibration

Geologic conditions such as strength of rock, the degree of weathering and type of lineation influence on wave propagation.

Similar investigations were conducted in the same rock over a certain area to determine whether amplitudes and attenuation rates were related.

Propagation of vibration

In general, the propagation law has the form:

$$V = K (D/W^b)^{-n} \quad (2)$$

Where V : peak partical velocity(P.P.V)  
 K : peak partical velocity intercept  
 D : Distance from blast-to-structure  
 W : charge weight per delay  
 b and n : exponents

According to extensive research carried out by U.S.B.M. and many other researchers. The blasting patterns have been divided into four kinds, open cut by bottom blasting, open cut by bench blasting, tunnel center cut and tunnel cut by caving blasting.

Results of measuring blast-to-induced ground vibration and corresponding site constants. Empirical and corresponding site constants. Empirical methods have been used to estimated value of b and n.

The measured data showed that cube root scaled distance was most applicable to this study. The typical vibration constants are estimated to b 1.60 to 1.78 for n and 43 to 138 for k in the granite base, while 1.5 for n and 17 to 87 for k in the gneiss base.

Seoul Metro-Subway Empirical formula

For Granite :  $V = KW^{0.57}D^{-1.75}$   
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S<sub>c</sub> : Compressive St. kg/cm<sup>2</sup>

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R : Rock Coefficiency  
 Seoul Granite = 0.0371  
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Explosive ; Emulsion  
Electric caps ; M/S  
Pattern ; Bench cut Tunnel

## Conclusion

Generally speaking, the geological feature of city Seoul is largely shaped by the Han River, which flows from the east to the west. The northern area of Seoul is comprised by Seoul Granite of Jurassic period and the Southern part by Gneiss of Cambrian period.

Therefore we should be alert for the vibration and noise caused by cautious blasting works under the minus 30 meters from the surface of the shopping and housing Zones.

Gneiss and Granite are generally regarded as stable rock. However in the vicinity of rivers a partial area is composed by Fractured Rock.

First of all, by applies Deck charge, empty line drilling, the pit of shaft construction is performed to eliminate or reduce vibration and noise.

Secondly, the tunnel pattern is based on experimental scale distance equation and crown part of tunnel face adopted pre-splitting and center cut of face also applied large bit diameter  $\phi 70\text{mm}$ - $120\text{mm}$  by burn cut. So the allowable vibration value is fixed not over 0.4 kine and noise not over 80db. We gauged each fire by instancel blastmate DS 277. Thus the Seoul city sub-way construction has been executed in a satisfactory manner.

By an analysis of the measured data near Seoul subway line, cube root scaling might be more reasonable than square root scaling.

To estimate the safe charge weight easily in the field, some nomograms and tables were given.

The Magnitude of ground vibrations can be reduced further, first, by using explosives that have low density and low velocity of detonation such as Kovex; second, by adopting three or four stage deck charging; third by using with sequential blasting machine at subway tunnel works.

For further understanding about the effects of explosives, rock strength and blasting types on the vibration levels it is necessary to carry out more tests.

Nowadays, we are proud so efficiency work with Sequential Blasting machine at subway Tunnel Works but I expect to apply Mac's auto marking System at Tunnel face sooner and hope to meet Robot Drill Semi-Jumbo( $13 \times 10$  meter face) presentation in the near future.

Table 1 Allowable value of blasting vibration

Classification	Vibration value on ground(cm/sec)
Cultural treasure	0.2
Housing apartment with partial Crack	0.5
Shopping center	1.0
Factory & reinforced concrete building	1.0~4.0

Table 2 Construction Schedule of Seoul Metro-Subway Line.

Division	Total	Line 1	line 2(Loop I )		Line 3	Line 4	Line 5	Line 6	Line 7	Line 8
			Main	Branch						
Length in Kilometer	278.6	9.5	48.8	5.3	27	30	55.8	35.2	47	20
Station	250	9	43	3	23	24	51	38	42	17
Rolling Stokes	844	96	272		132	138	76	46	62	22
Headway in Minute		3	3.5		3.5	3.5	2.5	5.5	3.0	3.5
Construction Terms	71-20	71-74	78-83	78-83	80-86	80-86	96-99	96-20	96-20	96-99
Passenger per Day in million	5	1	1.5		1.2	1.3				
Construction Status		run	run	run	60%	60%	run	80%	80%	run
Construction costs in 100 million won	23,579	330	9,440		6,548	7,277				

Table 3 Physical and Mechanical properties of rocks

Location	Rock type gravity(gr)	Specific gravity(gr)	Wave velocity(km/sec)		Compressive Strength(MPa)	Tensile strength(MPa)	Young's modulus( $\times 10$ )	Poasson's ratio
			P-wave	S-wave				
A	Gneiss	2.67	5.9	2.0	85	1.5	7.65	0.14
B	Granite	2.53	3.8	2.1	35	3.5	1.9	0.21
C	Granite	2.56	4.2	2.3	78	8.2	3.4	0.24
D	Granite	2.53	4.6	2.5	123	11.0	4.2	0.29
E	Gneiss	2.72	4.9	2.7	38	2.7	1.8	0.24
F	Granite	2.55	4.0	2.0	88	6.2	2.8	0.32
		2.57	4.9	2.6	12	2.4	2.4	0.2
G	Granite	2.57	4.9	2.6	145	11.0	3.5	0.22
H	Granite	2.54	3.9	2.2	39	2.7	0.7	0.17
I	Granite	2.49	3.4	1.8	15	2.5	0.55	0.22
J	Gneiss	2.68	4.6	2.6	140	17.5	2.35	0.27

Symbol; A : Bakseog gogae B : Hongeundong C : Jangchungdong  
 D : Keumhodong E : Woomindong F : Miari  
 G : Samsungyo H : Toegyero I : Toegyero  
 J : Dongjadong K : Seoul station

Table 4 Standardization in tunneling

	I	II	III	IV	V
Rock Kind	Stable rock	Moderately jointed and hard stratified or schistose	Fractured and friable rock	Unstable plastic & squeezing rock	highly Plastic squeezing & swelling ground
Burden (cm) Bit Gage = 38m	60	65	70	80	
Drilling	Full face	top heading & bench	Top heading & bench	line-drilling (pilot drift & bench)	for pilling
Support	Occasional Rock bolt	S.C., W.M. Systematic R.B.for cap	S.C., W.M. R.B. for cap & wall	S.C., W.M. R.B. & Steel Rib	S.C., W.M.F.P., Steel lagging & S.C. invert

S.C = Shotcrete R.B = Rock bolt  
 W.M = Wire Mesh F.P. = For Pilling



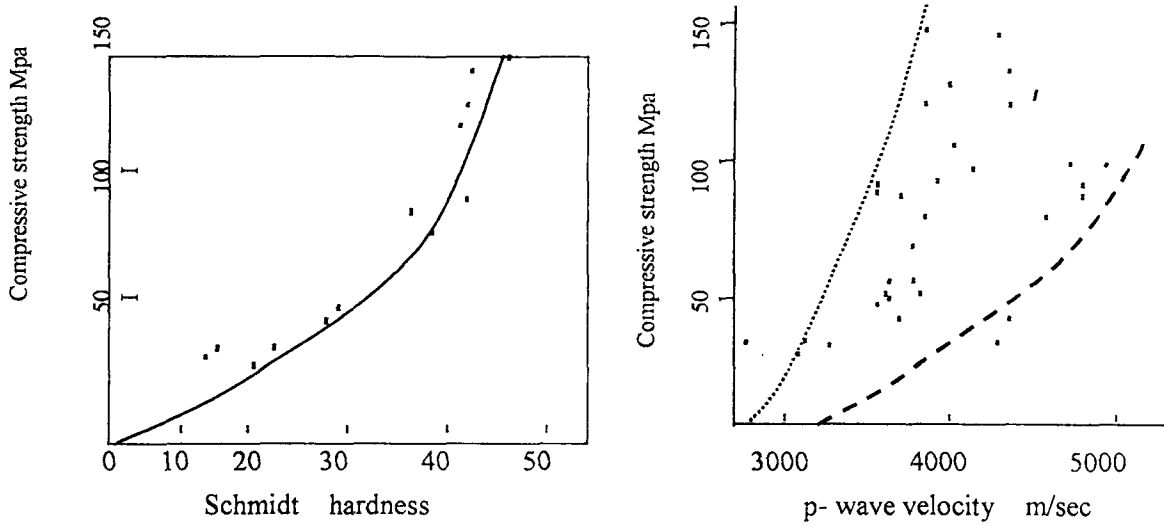


Fig. 1 Relation between compressive strength and Schmidt rebound  
 Fig. 2 Relation between compressive strength and P-wave velocity

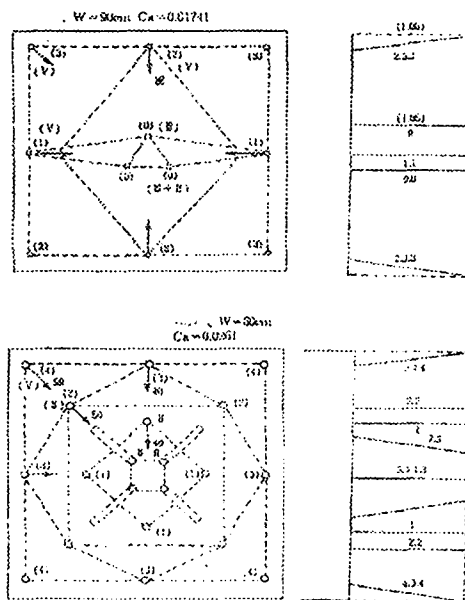


Fig. 3 Tunnel Blasting Drilling Pattern

**SEOUL -SUBWAY 6-9 SITE**  
**CHARGE CALCULATION**  
**Stable Rock**

DRILLED DEPTH 1.0m ADVANCE 0.8 m DETONATOR 388 pcs  
 CHARGE 96.515 kg NUMBER OF HOLES 570 m SPECIFIC DETONATOR 4.96PCS m<sup>3</sup>  
 TOTAL DRILLED DEPTH 678 m SPECIFIC DRILLED DEPTH 8.66m/ m<sup>3</sup> FACE AREA 97.877m<sup>2</sup>  
 SPECIFIC CHARGE 1.233kg/m<sup>3</sup> EXCAVATED SOLD ROCK OF FULL FACE 78.30 m<sup>3</sup>

Face	Round	Classification	Cap No.	No. of Holes	Amount of charge						Remarks (Drilled depth) m		
					Slurry(k-100)		F-1		F - 2			Total	
					φ 25 mm X 433 mm X 250 g/pc		φ 17 mm X 425 mm X 100g/pc		φ mm X mm X g/pc				
					Per hole	Sub -total	Per hole	Sub total	Per hole	Sub Total			
Hole	PCS	PCS	PCS	PCS	PC	PCS	g						
A (Top Gallery Right Face)	1	Cut Holes	MS 1~ 4	4	1 1/3	5.34					1,335		
		Cut Spreader H.	5~12	12	1 1/3	16					4,000		
		Stopping H.	13~20	26	1	26						6,500	
			DS 6~12	41	1	41						10,250	
		F-1 Line	13~15	21	1/4	5.25	1	21				3,412.5	
		Stopping H.	16~17	12	1	12						3,000	
		Foot H.	18~20	12	1 1/3	16						4,000	
		Empty H. (cut)	-	(15)	-	-	-	-	-	-	-	-	∅120mmX 2'
		空孔(stopping)	-	(53)	-	-	-	-	-	-	-	-	∅120mmX 1"
		Line drilling	-	(35)	-	-	-	-	-	-	-	-	∅38mmX 2'
Sub Total	-	(103)			121.59 pcs		21pcs			32,497.5g			
	-	128					2,100g			32.4975kg			
	-	231			30,397.5 g								
B Top Gallery Left Face	2	Stopping H.	MS1~20	76	1	76					19,000		
			OS 6~9	19	1	19						4,750	
		F-1 Line	10~16	36	1/4	9	1	36				5,850	
		Foot H.	17~20	11	1 1/3	14.67						3,667.5	
		空孔(stopping)	-	(21)	-	-	-	-	-	-	-	-	∅120mmX 1m
		Line drilling	-	(58)	-	-	-	-	-	-	-	-	∅38mmX 2m
		Sub Total;		(79)			118.67 pcs		36pcs			33,267.5g	
		142					3,600g			33.2675kg			
		221			29,667.5g								
C Top gallery		Total		(182)		240.26 pcs		57pcs			65,765g		
				270									
				452		60,065g		5,700g			65,765kg		
D Bench	3	Stopping H.	MS1~20	65	1	65					16,250		
			DS 6~11	22	1	22						5,500	
		F-1 Lines	12~13	10	2/5	4	1	10				2,000	
		Foot H.	14~19	21	1 1/3	28						7,000	
		Total		118			119pcs		10pcs			30,750g	
				118				1000g			30.75kg		
E Full Face		Gross Total		(182)		359.26 pcs		67pcs			96,515g		
				388									
				570		89,815g		6,700g			96.515kg		

Seoul Subway 6-9 Site (新客車) By Dr. Gwan Huh  
 Drilling & Ignition Pattern S = 1 : 100  
 Stable Rock Unit : cm

Burden : 60cm  
 Spacing : 80cm  
 Empty Holes :  $\phi 120\text{mm}$   
 Charge Hole :  $\phi 100\text{mm}$   
 DSD site : ASD

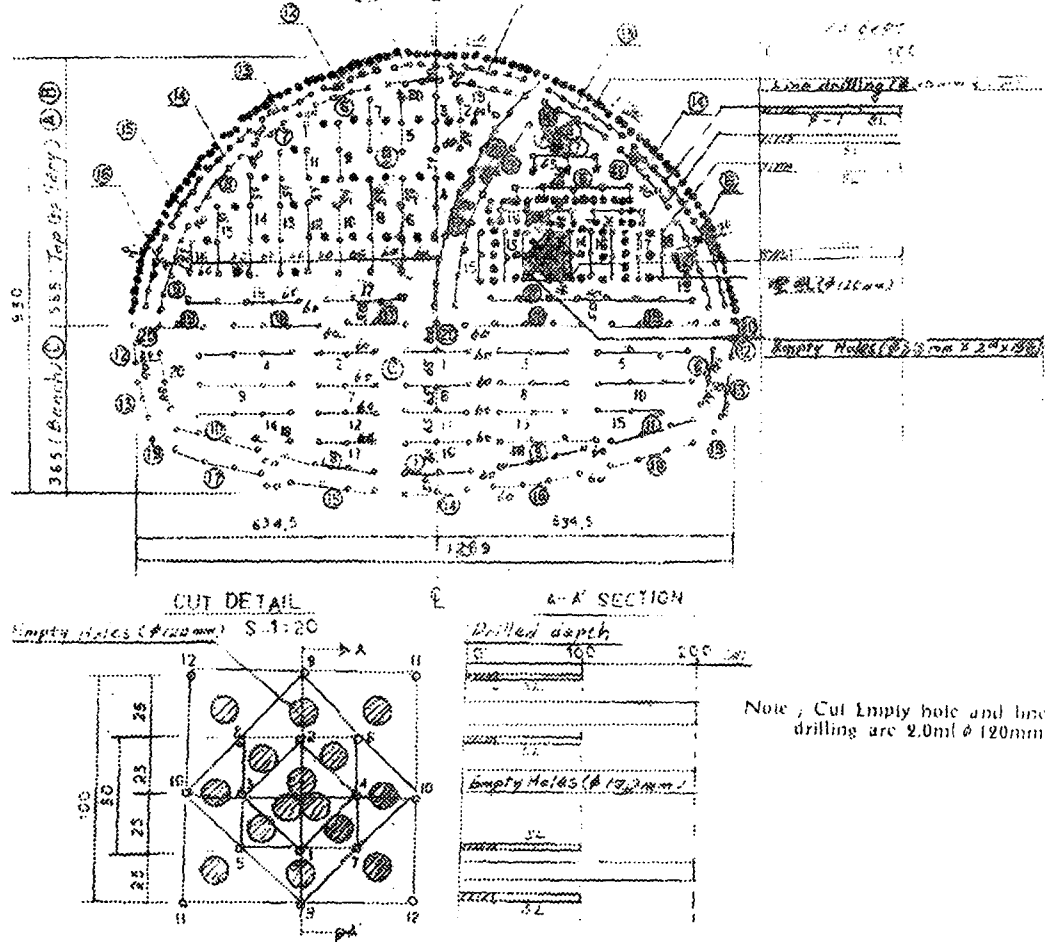


Fig 4 Pattern