# Theoretical Approach to Calculate Surface Chloride Content C<sub>s</sub> of Submerged Concrete under Sea Water Laden Environment

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#### **ABSTRACT**

The ingress of chloride ions plays a crucial role for service life design of reinforced concrete structures. In view of durability design of concrete structures under marine environment, one of the most essential parameters is the surface chloride content of concrete. However, on the basis of the results of in-situ investigation, this value has been determining in the numerous studies on the durability design of concrete structures. Hence, it is necessary to confirm the range of the surface chloride content in order to establish a unified durability design system of concrete.

This study suggests a rational and practical way to calculate the maximum surface chloride content of submerged concrete under marine environment. This approach starts with the calculation of the amount of chloride ingredients in normal sea water. The capillary pore structure is modeled by numerical simulation model HYMOSTRUC and it is assumed to be completely saturated by the salt ingredients of sea water. In order to validate this approach, the total chloride content of the mortar and concrete slim disc specimen was measured after the immersion into the artificial sea water solution. Additionally, the theoretical, the experimental and in-situ investigation results of other researchers are compiled and analyzed.

Based on this approach, it will follow to calculate the maximum surface chloride content of concrete at tidal zone, where the environment can be considered as a condition of dry-wetting cycles.

#### 1. Introduction

The deterioration of concrete structures due to chloride diffusion is now a growing problem in many countries. It is well known that the main cause of reinforcement corrosion of concrete exposed to marine environment is chloride diffusion. Therefore, the durability design of concrete structures under marine environment is regarded as a prerequisite in order to create durable concrete and it is also a criterion for the mix proportion and cover depth optimization.

The literatures have been flooded with methodological approaches to carry out the durability design of concrete, since 1980s. In the process of durability design, the surface chloride content,  $C_{\rm s}$ , has been regarded as one of the crucial parameters. At the same time, the surface chloride content is considered as a standard parameter to define environmental degree of chloride laden concrete. However, many researchers and engineers have determined this value empirically. Because it is not easy to define the value due to time variation with elapsed time, environmental condition, and especially microclimatic condition at concrete surface. In this study, a rational and practical way is suggested for the calculation of the maximum  $C_s$  in the submerged portion.

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### 2. Significance and Approach to Calculate $C_s$

 $C_s$  depends on many parameters such as the ingredient of sea water, the geographical location, and the exposure condition with water level. Many researchers have confirmed this value, based on the result of in-situ investigation of old concrete structure.

The concrete sections exposed to marine environment are mainly divided into three parts: atmospheric zone, tidal zone, submerged zone. The origin chloride sources of concrete are air borne chloride carried from sea for atmospheric zone, brackish water or sea water for tidal zone, and sea water for submerged zone, respectively. For this reason, equivalent chloride concentration,  $C_{\text{env}}$  can be expressed as maximum equivalent surface chloride content which is described by Eq. (1).

$$C_{\text{env}} = C_{\text{sea}} f(h)$$
 (1)

In the tidal zone where intermittent drying-wetting cycles occur, chloride ingredients can be accumulated at the surface of concrete if sea water is dried out. The evaporation can increase  $C_s$  of concrete in the tidal zone to a higher value than the chloride content of sea water. In submerged zone,  $C_s$  does not increase infinitely because the value is governed by the degree of saturation. That is, the  $C_s$  initially will fluctuate about a steadily rising level and then increase with elapsed time slowly and continuously. Finally, this value will stagnate if it equals the concentration of sea water approximately. Maximum  $C_s$  can be regarded as an imperative parameter to predict a service life of concrete for long term and a constant value after a certain short period has elapsed. Thus, in order to characterize the chloride build-up, the rational expression can be depicted by [1]:

$$C_{s}(t) = C_{max} \left( 1 - e^{-at} \right) \tag{2}$$

 $C_s$  depends on the amount of chloride ingredient in sea water. Table 1 shows the major constituents of normal sea water. The amount of competitive salt can be calculated as:

$$C_c = Q_e W_e \tag{3}$$

NaCl, MgCl<sub>2</sub> are mainly considered in this study, since they are the major competitive salts of sea water. The quantity of NaCl, MaCl<sub>2</sub> can be calculated as 27.47 g/l, 3.82 g/l, respectively and the mass fraction,  $\Omega_c$ , can be obtained if total salt ingredients are assumed to be 35.0 %.

If it is assumed that concrete is completely saturated with sea water, the chloride content in concrete can be calculated theoretically. To begin with, all the capillary pores are assumed to be filled with sea water and the salt quantity  $(Q_c)$  is calculated by the following equation:

$$Q_c = \Omega_c \ \rho_c \ \phi_{cap} \tag{4}$$

The value should be translated into a quantity of total Cl to consider an atomic weight of ingredients of salt as:

$$Q_c^{CI} = Q_c \frac{W_{CI}}{(W_c + W_{CI})} \tag{5}$$

Total quantity of Cl composition can be simply calculated as:

$$Q_{Total}^{CI} = Q_{N\alpha CI}^{CI} + Q_{MgCI_2}^{CI}$$

$$\tag{6}$$

Now, we can calculate how much the chloride would be accumulated at the surface of submerged concrete under seawater laden environment.

Table 1 Chemical element of sea water [2]

| Chemical Element (e) | C1  | Na  | Mg | SO <sub>4</sub> | K  | Ca | HCO <sub>3</sub> |
|----------------------|-----|-----|----|-----------------|----|----|------------------|
| Volume (m mol/ l)    | 548 | 470 | 54 | 28              | 10 | 10 | 2                |



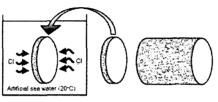


Fig. 1 Microstructure modeling

Fig. 2 Chloride immersion testing

Table 2 Mix pro. of mortar or con'c

Unit weight (kg/m³)

Water Cement Sand (Gravel

| Onit weight (kg/m/ |        |      |          |  |  |  |
|--------------------|--------|------|----------|--|--|--|
| Water              | Cement | Sand | (Gravel) |  |  |  |
| 185                | 411    | 706  | (1001)   |  |  |  |
| 185                | 370    | 720  | (1021)   |  |  |  |
| 185                | 336    | 732  | (1038)   |  |  |  |

) is not contained in mortar

#### 2. Experiment and Discussion

The  $C_s$  of the mortar disk and concrete disk is calculated and presented in Table 2.At the end of the 20 °C water curing period, the top 2 mm of the specimens was sawn and then immersed in artificial sea water with same composition as given in Table 1. Total chloride ion content in the specimens was measured in accordance with the technique described in ASTM C1152-97 and expressed as a weight ratio of chloride to concrete ( $g_{cl}$  /  $g_{conc}$ ) for mortar and concrete specimens.

Fig. 3 shows the calculation results of capillary pore according to the numerical simulation model HYMOSTRUC model. Because the HYMOSTRUC model can calculate the microstructural properties of hardened cement paste, these values should be converted to the capillary porosity of concrete to consider volumetric portion of cement, water, sand and coarse aggregate, respectively.

Fig. 4 and Fig. 5 present comparisons of measured  $C_s$  and calculated  $C_s$  in mortar and concrete, respectively. For concrete, accurate relationship between experiment and calculation is not

formulated because of skin effect and sporadic existence of coarse aggregates. However, it should be noticed that predicted values of  $C_{\rm s}$  are approximately in reasonable agreement with the measured values in case of mortar. More than anything else, maximum  $C_{\rm s}$  theoretically depends on w/c ratio because dense concretes have finer pore structures and should absorb less chloride in view of build-up rate of chloride, according to this result.

Fig. 6 depicts the result of comparison of this result and other studies.[3]~[6] As shown in this figure,  $C_s$  depends on w/c ratio but is mostly in range of 5.5 to  $7 \times 10^{-3}$  g<sub>Cl</sub> /g<sub>conc</sub>, which coincides with the result of in-situ investigation of Takeda and calculated result of Bazant approximately.

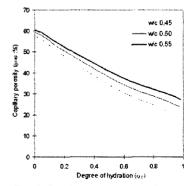


Fig. 3 Capillary pore with various stage of hydration

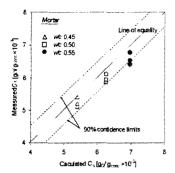


Fig. 4 Comparison of calculated and measured C<sub>s</sub> (in mortar)

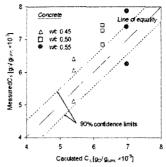


Fig. 5 Comparison of calculated and measured C<sub>s</sub> (in concrete)

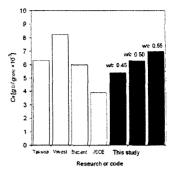


Fig. 6 Comparison of this result and other studies

#### 3. Conclusions

The accuracy of this approach is dependent on pore structural properties of concrete and the microclimatic condition at near surface concrete.  $C_s$  is mainly an environmental parameter, however, that is also a little governed by material properties of concrete such as w/c ratio, weight of cement, and the type of cement. For these reasons, it is not easy to define this value of  $C_s$  in concrete deterministically.

This approach is theoretically based on the ingredient and amount of seawater, and microstructural characteristics of cementious materials. This result can be used for reasonable durability design of concrete structure. On the basis of the approach suggested, the calculation of  $C_s$  in tidal concrete will be followed.

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## Notations

| Cmax: maximum surface chloride content,                                  | f(h): function of height above tidal zone,                    |  |  |  |
|--|---|--|--|--|
| C <sub>sea</sub> : concentration of chlorides in the seawater,           | $Q_e$ : quantity of chemical element $e$ ,                    |  |  |  |
| C <sub>env</sub> : equivalent chloride concentration,                    | $Q_{\epsilon}^{ck}$ : quantity of Cl in competitive salts $c$ |  |  |  |
| $\Omega_{\epsilon}$ : mass fraction of competitive salt $c$ of seawater, | a: rate of coefficient of chloride accumulation,              |  |  |  |
| $\phi_{cap}$ : volume of capillary pores,                                | $W_e$ : atomic weight of element e,                           |  |  |  |
| t : elapsed time,  | $\rho_c$ : density of the competitive salt $c$ of seawater    |  |  |  |