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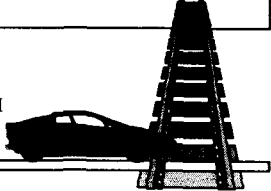
차량추종 모델과 운전자 행태에 근거한 철도교차로의 동적 딜레마 구간

문 영 준

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**Car-Following and Dynamic Dilemma Zone Model
for Determining Gate Operation Times Based on
Driver Behavior at Highway-Rail Intersections**

Ph.D Thesis
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CE/UIUC, 1997



MyJ 1

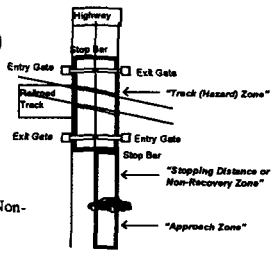
Outline

- Introduction
- Coleman & Moon Methodology for Designing Four Quad-Gate (QG) Operation Times
- Driver Behavior at Highway-Rail Intersections
- Development of New Methodology
 - Dynamic Dilemma Zone (DDZ) and Car-Following (CF)
 - Data Collection and Reduction
 - Field Data Analysis for Vehicular Speed Profiles
 - Simulation & Validation for DDZ Estimation
 - Optimal Gate Operation Times
- Conclusions and Recommendations

MyJ 2

1 Introduction

- Four Quadrant Gate (QG) at Highway-Rail Intersections (HRI)
 - Gates
 - Entry Gate
 - Exit Gate
 - Zones
 - Approach Zone
 - Stopping Distance or Non-Recovery Zone
 - Track or Hazard Zone



MyJ 3

Introduction

- Determining Gate Operation Times
 - Coleman & Moon (TRR 1553, 1997)
 - Gate Delay, T_D using Dilemma Zone Concept
 - From Research on Traffic Signal Intersections
 - Single Vehicle and Constant Speed Approach
 - Gate Interval Time, T_I
 - Using Predetermined Constant Minimum Speed
 - Higher Range of Interval Time (14 - 22 sec)
 - Possibility of Crossing Violation during T_I

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Problem Statement

- Assumptions on Static Dilemma Zone
 - Single Vehicle
 - Constant Speed Approach
- Questions?
 - Platoon Approach
 - Speed Variation
 - Gate Operation Parameters

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Problem Statement

- Factors affecting Gate Operation Times
 - Driver Behavior:
 - Stopping, Clearance, and Continuation Distance
 - Approach Speed, Perception-Reaction Time (PRT), Accel/Decel Rates
 - Geometry:
 - Width: No. of Railroad Tracks, App. Highway Lanes
 - Distance between Stop Bar and Gate
 - Others:
 - Warning Devices, Roughness, Obstacles, etc.

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Objectives

- Dynamic Dilemma Zone (DDZ) Model
 - Existence
 - By Car-Following Logic at HRI
- QG Operating Parameters
 - Minimize the risk of trapping a vehicle between the entry and exit gates
- Driver Behavior at HRI
 - Speed Profiles, Accel/Decel Rates, and Headway
 - Using Data Collection and Analysis
 - Input for a simulation model to DDZ

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7

2 Design of QG Operation Times

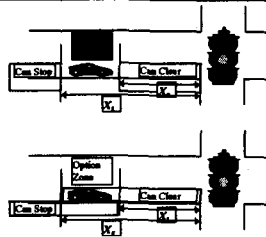
- Gate Operation Times
 - Gate Delay
 - Interval between initiation of flashing light and the entry gate arm descent
 - 3 seconds (MUTCD), 3 - 5 seconds (IDOT)
 - Gate Interval Time
 - Interval between initiation of the entry and exit gates descent
 - 1 - 3 sec min. (LRT), 3 - 5 sec (NYSDOT)
- Coleman & Moon (*TRR 1553, 1997*)
 - Design Approach by Dilemma Zone Concept

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8

Dilemma Zone Concept

- Application to Highway-Highway Intersections (HHI)
- The Dilemma:
 - The drivers' decision *to proceed* through the intersection or *to stop* when the signal changes from green to yellow

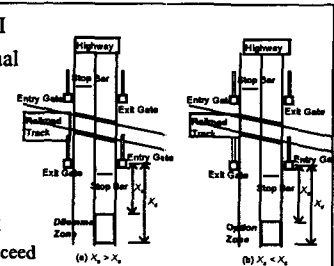


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9

Dilemma Zones at HRI

- Similarity to HHI
- Drivers Use Visual Signal
 - Flashing Lights
 - Gates
 - Quad Gates
 - Dual Gates
- Decision Making
 - To stop or to proceed
 - Dilemma Zone



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10

Distance Variables

- Stopping Distance (X_s)

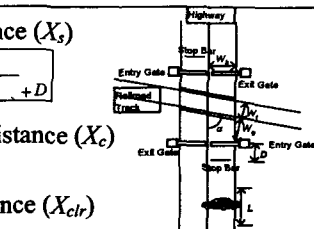
$$X_s = \Delta T \cdot v + \frac{v^2}{2a} + D$$

- Continuation Distance (X_c)

$$X_c = v \cdot (W_{cr} - L)$$

- Clearance Distance (X_{clr})

$$X_{clr} = W_{cr} - L$$



$$W_{cr} = \frac{W}{\sin \alpha} + \frac{2 \cdot W_r}{\tan \alpha} + \frac{2 \cdot W_e}{\sin \alpha}, \quad \alpha \leq 90^\circ$$

$$= \frac{W}{\sin \alpha} + \frac{2 \cdot W_r}{\tan \alpha} + \frac{2 \cdot W_e}{\sin \alpha}, \quad \alpha > 90^\circ$$

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11

Safe Decision Location

- $X_s = X_c$: Safe Decision Location
 - Both dilemma and option zones are eliminated
 - A point or distance is obtained which assures the likelihood of drivers stopping or clearing the intersection
 - $X_s > X_c > X_c$: Dilemma Zone
 - $X_s < X_c < X_c$: Option Zone
- This gives:

$$TV = \left\{ \Delta T + \frac{v}{a} + \frac{D}{v} \right\} + \left\{ T \cdot (W_{cr} - L) \right\}$$

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12

QG Operation Times

Gate Operation Time: T_G

- Gate Delay, T_D
- Gate Interval Time, T_I

$$T_G = T_D + T_I,$$

$$T_D = \left\{ \Delta T + \frac{v}{2(d + G \cdot g)} + \frac{D}{v} \right\},$$

$$T_I = \left\{ \frac{1}{v} (W_{ght} + L) \right\}$$

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13

Findings for Six Intersections

Gate Delay

Street of the Intersection, City	Approach Speed, kph (mph)	Gate Delay at $dT(\text{RT}) = 1$	Gate delay at $dT(\text{RT}) = 2.5$
U.S. Route 136, McLean	72 (45)	4.5	6.0
N. Grand Ave., Springfield	56 (35)	3.7	5.2
Hawthorn St., Hartford	64 (40)	4.1	5.6
Main St., Gardner	56 (35)	3.7	5.2
Main St., Pontiac	40 (25)	3.1	4.6
Trunk Route 35A, Chenoa	40 (25)	3.1	4.6

Gate Interval Time

Street of the Intersection, City	Min. Spd in Track Zone, kph (mph)	Gate Interval for Auto	Gate Interval for Truck
U.S. Route 136, McLean	8 (5)	8.9	15.1
N. Grand Ave., Springfield	8 (5)	14.3	29.5
Hawthorn St., Hartford	8 (5)	14.7	21.1
Main St., Gardner	5 (3)	11.4	21.9
Main St., Pontiac	5 (3)	11.8	22.3
Trunk Route 35A, Chenoa	8 (5)	7.0	13.3

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14

3 Driver Behavior at HRI

Probability of Crossing

- Meeker and Barr (1989), Meeker et al. (1997)
- As a function of train location, train speed, time available until train arrival
- A significant percentage of drivers made a decision to cross the intersection during activation of the flashing lights

Head Movements

- Sanders (1976), Aberg (1988), Tenkink and Horst (1988)
- Performed Action: Head Movements, Lowering Windows, Reducing Speed
- Fewer head movements at higher speed of vehicles

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15

Driver Behavior at HRI

Speed Reduction

- Shinar and Raz (1982)
 - Observations: 200 and 80 meters from the track (no trains)
 - Most drivers reduced speed under 5 different protection conditions
- Meeker et al. (1997)
 - 60 drivers under two different protection conditions
 - Driver behavior at train's approaching

Driver Behavior	Flashing Lights Only	Gates and Flashing Lights
Cross despite Warning	39 of 58 (67%)	23 of 60 (38%)
Stop before Proceeding	14 of 39 (36%)	4 of 23 (17%)
Slow before Proceeding	20 of 39 (51%)	7 of 23 (30%)
Did not Stop or Slow	3 of 39 (8%)	12 of 23 (52%)

- Summary: Drivers reduced their approach speed most at intersections regardless of trains approaching

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16

Speed Reduction as a Key Factor

Intergreen Interval at HHI

- Chang et al. (1985), Horst and Wilmink (1986)
- The duration of yellow is dependent on vehicle speed
- Shortcomings: constant speed of single vehicle

Gate Operation Times at HRI

- Coleman and Moon (1997)
- Different assumption on speed profile at HHI
- Constant speed approach for gate delay
- Critical min. speed at track zone for gate interval

Speed Reduction as a Key Variable to Represent Driver Behavior at HRI

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17

4 New Methodology

Dynamic Method for Defining Variables

Speed Variation

$$v(t) = v(t-1) + \frac{T}{(a(t-1) + a(t))}$$

Vehicle Location

$$x(t) = x(t-1) + \frac{T}{v(t-1) + v(t)}$$

Stopping Dist.

$$X_c(t) = \Delta T \cdot v(t) + \frac{v^2(t)}{2a} + D$$

Clearance Dist.

$$X_c^* = \frac{T}{v(t)} + \frac{T}{v(t)} + \frac{T}{v(t)} + \dots + \frac{T}{v(t^*)} + \frac{T}{v(t^*)}$$

Continuation Dist.

$$X_c(t) = \frac{T}{v(t-1) + v(t)}$$

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18

Remaining Distance

- Remaining Distance = Safe Stopping Dist. - Vehicle Location
- $X_r(t') = X_s(0) - x(t')$
- $x(0) = 0, x(T_D) = X_s(0)$
- $X_r(0) = X_s(0), X_r(T_D) = 0$

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Remaining Dist. Vs. Vehicle Location

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Dynamic Dilemma Zone (DDZ)

- DDZ = Remaining Dist. - Continuation Dist.
- DDZ = $X_c(t') - X_s(t')$, for $t' = 0, 1, \dots, T_D$
- Eliminating Dynamic Dilemma Zone at any t'
- At $t' = 0$, Remaining Dist. = Safe Stopping Dist.
- Optimal Gate Operation Times
- Min. DDZ for Constant Speed Approach
- Min. DDZ for Speed Reduction

$$\text{Max } T_G = T_D + T_I$$

$$\text{s.t. } X_c(t') = X_r(t') \leq X_s, \quad t' = 0, 1, \dots, T_D.$$

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DDZ for Constant Speed

- Constant Speed over time t'
- $v(t') = v, a(t') = 0$, for $t' = 0, 1, \dots, T_D$
- DDZ for Const Spd $X_c(t') = v \cdot (T_D - t')$, $t' = 0, 1, \dots, T_D$
- $X_r(t') = X_s(0) - x(t')$
- $= X_s - (t' \cdot v)$, $t' = 0, 1, \dots, T_D$.
- Optimal Gate Delay (= Coleman&Moon Method)

$$T_D = \frac{1}{v} \cdot X_s = \frac{1}{v} \cdot \left\{ \Delta T \cdot v + \frac{v^2}{2 \cdot (a + G \cdot g)} + D \right\}$$

$$= \Delta T + \frac{v}{2 \cdot (a + G \cdot g)} + \frac{D}{v}$$

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DDZ for Constant Speed

- Optimal Gate Interval Time
- $X_{ctr} = T_G \cdot v = (T_D + T_I) v$
- $X_{ctr} = X_s + (W_{ght} + L)$
- thus, $(T_D + T_I) v = X_s + (W_{ght} + L)$
- This gives

$$T_I = \frac{1}{v} \cdot (W_{ght} + L)$$

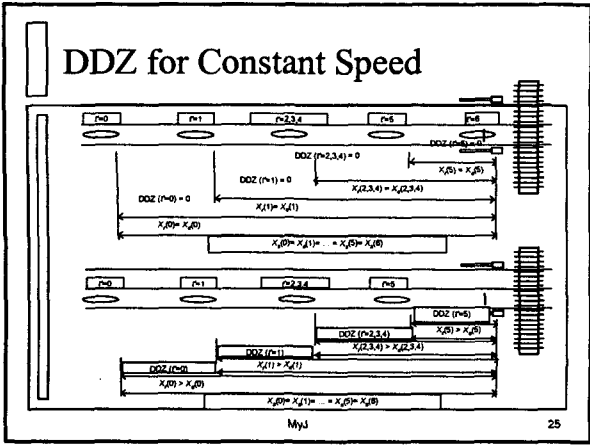
- Identical to Coleman&Moon Method

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DDZ for Constant Speed

	$T_D = 6 \text{ sec}$ Based on eq. (4.10)	$T_D = 5 \text{ sec}$	$T_D = 7 \text{ sec}$
Assumptions			
Approach Speed, v	21 m/s	21 m/s	21 m/s
FRT	2.5 sec	2.5 sec	2.5 sec
Typical Decel., d	3.0 m/s ²	3.0 m/s ²	3.0 m/s ²
Stopp. Distance, X_s	126 meter	126 meter	126 meter
Track Width	42 meter	42 meter	42 meter
Dynamic Dilemma Zone			
$t' = 0 \text{ sec.}$	DDZ = $X_s - X_c$ 0 = 126 - 126	DDZ = $X_s - X_c$ 21 = 126 - 105	DDZ = $X_s - X_c$ -21 = 126 - 147
$t' = 1 \text{ sec.}$	0 = 105 - 105	21 = 105 - 84	-21 = 105 - 126
$t' = 2 \text{ sec.}$	0 = 84 - 84	21 = 84 - 63	-21 = 84 - 105
$t' = 3 \text{ sec.}$	0 = 63 - 63	21 = 63 - 42	-21 = 63 - 84
$t' = 4 \text{ sec.}$	0 = 42 - 42	21 = 42 - 21	-21 = 42 - 63
$t' = 5 \text{ sec.}$	0 = 21 - 21	21 = 21 - 0	-21 = 21 - 42
$t' = 6 \text{ sec.}$	0 = 0 - 0		-21 = 0 - 21
	Maximized Dynamic Dilemma Zone	Dynamic Dilemma Zone	Dynamic Option Zone
Gate Interval Calculation			
Track Zone Speed	21 m/s	The vehicle may hit the entry gate	21 m/s
T_I	2.0 seconds		2.0 seconds

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DDZ for Speed Reduction

- Speed Reduction over time t'
 - $v(t') = v, a(t') = a$ (constant decel.), $t' = 0, 1, \dots, T_D$
 - $v(t') = v(t' - 1) + a$
 - $x(t') = t' \cdot v + t'^2/2 \cdot a$
 - $X_s(t') = X_s - x(t') = X_s - t' \cdot v - t'^2/2 \cdot a$
 - $X_f(t') = X_f(t' - 1) - \{v + a(2t' - 1)/2\}$
- At $t' = T_D$
 - $X_s(T_D) = X_s - x(T_D) = X_s - T_D \cdot v - T_D^2/2 \cdot a$
 - $X_f(T_D) = 0$
 - $X_s(T_D) = X_f(T_D)$

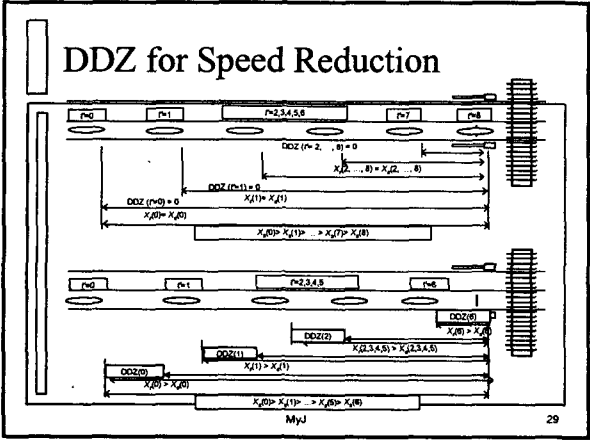
DDZ for Speed Reduction

- Optimal Gate Delay

$$T_D = -\frac{v}{a} \pm \sqrt{\left(\frac{v}{a}\right)^2 + \frac{2 \cdot X_s}{a}}$$
- Optimal Gate Interval Time
 - By $T_i = \frac{(W_{gr} + L)}{v(T_D)}$
 - Then $T_i = -\frac{(v + T_D \cdot a)}{a} \pm \sqrt{\left(\frac{v + T_D \cdot a}{a}\right)^2 + \frac{2 \cdot (W_{gr} + L)}{a}}$
 - $= -\frac{v(T_D)}{a} \pm \sqrt{\left(\frac{v(T_D)}{a}\right)^2 + \frac{2 \cdot (W_{gr} + L)}{a}}$

DDZ for Speed Reduction

	$T_D = 8$ sec Based on eq. (4.13)	$T_D = 6$ sec By Table 4.1
Assumptions	21 m/s -1.31 m/s ² 126 meter 42 meter	21 m/s -1.31 m/s ² 126 meter 42 meter
Dynamic Dilemma Zone	$DDZ = X_s - X_f$	$DDZ = X_s - X_f$
$t' = 0$ sec.	0 = 126.08 - 126.08	23.66 = 126.08 - 102.42
$t' = 1$ sec.	0 = 105.73 - 105.73	23.66 = 105.73 - 82.07
$t' = 2$ sec.	0 = 86.70 - 86.70	23.66 = 86.70 - 63.04
$t' = 3$ sec.	0 = 68.97 - 68.97	23.66 = 68.97 - 45.31
$t' = 4$ sec.	0 = 52.56 - 52.56	23.66 = 52.56 - 28.90
$t' = 5$ sec.	0 = 37.45 - 37.45	23.66 = 37.45 - 13.79
$t' = 6$ sec.	0 = 23.66 - 23.66	23.66 = 23.66 - 0.0
$t' = 7$ sec.	0 = 11.17 - 11.17	
$t' = 8$ sec.	0 = 0 - 0	
Gate Interval Calculation	10.52 m/s 4.0 seconds 7.5 seconds	The vehicle may hit the entry gate.



Car-Following DDZ

- 5th GM Model
 - $\dot{x}^{n+1}(t+1) = T \cdot \dot{x}^{n+1}(t) + [x^n(t) - x^{n+1}(t)]$ (Acceleration/Deceleration)
 - $\dot{x}^{n+1}(t) = \dot{x}^{n+1}(t-1) + \frac{T}{2} [\dot{x}^{n+1}(t-1) + \dot{x}^{n+1}(t)]$ (Speed)
 - $x^{n+1}(t) = x^{n+1}(t-1) + T [\dot{x}^{n+1}(t-1) + \dot{x}^{n+1}(t)]$ (Distance)
- Stopping Dist. $X_s^{n+1}(t') = \Delta T \cdot \dot{x}^{n+1}(t') + \frac{\dot{x}^{n+1}(t')^2}{2a} + D$
- Continuation Dist. $X_c^{n+1}(t') = \frac{T}{2} \sum_{i=0}^{t'} [\dot{x}^{n+1}(i-1) + \dot{x}^{n+1}(i)]$
- Remaining Dist. $X_r^{n+1}(t') = X_s^{n+1}(t') - X_c^{n+1}(t')$

Car-Following DDZ

- DDZ = $X_{r,n+1}(t) - X_{s,n+1}(t)$, for $t' = 0, 1, \dots, T_D$

$$X_{c,n+1}(t') < X_{r,n+1}(t') < X_{s,n+1}(t')$$

where, $X_{r,n+1}(t') = X_{r,n+1}(0) - x_{n+1}(t')$, $t' = 0, 1, \dots, T_D$.

- Optimal Gate Operation Times

$$\text{Max } T_G = T_D + T_I$$

s.t. $X_{c,n+1}(t') = X_{r,n+1}(t') < X_{s,n+1}(t')$, $t' = 0, 1, \dots, T_D$.

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31

Data Collection and Reduction

- Site Selection

- Vehicle Arresting Barrier (VAB) Installation
 - Hartford, McLean, and Chenoa Sites
- Considering AADT for Car-Following
- Two Sites: Hartford and McLean

- Data Collection

- Single day in Oct. 1996 & in Jul. 1997 for each site
 - weather: partly cloudy
 - morning (2 hrs) and afternoon (2 hrs)
- Video Camera
 - viewing area: about 100 - 130 meters (300 - 400 ft)

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32

Data Collection and Reduction

- Reference Points

- Hartford: 77 and 31 meters from the entry gate
 - Zone A: 46 meters long (from 77 to 31 meters)
 - Zone B: 31 meters long (from 31 meters to the entry gate)
 - Zone C: 26 meters long (from the entry to the exit gates)
- McLean: 93 and 31 meters from the entry gate
 - Zone A: 62 meters long (from 93 to 31 meters)
 - Zone B: 31 meters long (from 31 meters to the entry gate)
 - Zone C: 14 meters long (from the entry to the exit gates)

- Camera Location:

- A good view of approach zone and the track zone

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33

Data Collection and Reduction

- Sample Size

- If 100 speed data are collected
 - 4.25 m/s of std. dev. and
 - 1.00 m/s permissible error
 - 95% confidence

$$N = \left(\frac{t_{\alpha/2} \cdot s}{e} \right)^2 = \left(\frac{1.96 \times 4.25}{1.00} \right)^2 \approx 70$$

- Grouping of Data

- Single vs. Platoon
- Weber-Fechner Relationship
 - average 10 m/s of speed in the track zone
 - 4.6 meters of buffer dist. (B) recommended
 - average 4.6 sec of headway

$$h = \Delta t + \frac{(L+B)}{v}$$

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34

Data Collection and Reduction

- Data Reduction

- Time Unit: 1 sec (collection) \leftrightarrow 1/30 sec (reduction)
- Video Tape Recorder \leftrightarrow Spreadsheet (MS Excel)
- Manual data recording method instead of image processing technique

- Getting the values

- Speed = dist. / veh. travel time for each section
- Accel (decel) rate = speed variation between two sections / travel time for these two sections
- Headway = interarrival time of vehicles

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35

Data Collection and Reduction

- Summary of Data Reduction

- Oct. 1996

Approach Type	Single Vehicles						Platoon		
	Hartford		McLean		Hartford		McLean		
Site Name	AM (1)	PM (2)	AM (1)	PM (2)	AM (1)	PM (2)	AM (1)	PM (2)	
Time Period (hr)	8	5	8	5	8	5	8	5	
Dist. Group	WB	WB	BB	-	WB	WB	BB	-	
Distance	60	115	106	-	31	44	21	-	
No. of Gates	80	115	106	-	71	254	44	-	
No. of Vehicles									
Vehicle Type									
Auto	56	141	66		56	117	34		
Truck	24	44	40		17	44	14		
BU*	15	34	21		11	56	7		
WB, 12"	6	8	19		6	10	7		
School Buses	0	0	2		0	1	0		

- Jul. 1997

Approach Type	Single Vehicles						Platoon		
	Hartford		McLean		Hartford		McLean		
Site Name	AM (1)	PM (2)	AM (1)	PM (2)	AM (1)	PM (2)	AM (1)	PM (2)	
Time Period (hr)	8	5	8	5	8	5	8	5	
Dist. Group	WB	WB	BB	WB	WB	WB	BB	WB	
Distance	142	200	81	111	64	62	13	28	
No. of Gates	142	200	81	111	172	137	32	106	
No. of Vehicles									
Vehicle Type									
Auto	74	117	53	100	96	85	23	67	
Truck	68	63	28	58	76	62	11	39	
BU*	33	40	21	35	39	29	6	31	
WB, 12"	35	23	7	23	37	33	5	8	
School Buses	0	0	0	0	0	0	0	0	

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36

Vehicular Speed Profiles for Field Data at HRI

- Microscopic Speed Profiles
 - A Primary Factor in the Designing and Controlling Traffic Performance on Highway
 - Estimate DDZ and Car-Following at HRI
- Two Groups for Speed Profiles
 - Single Vehicles
 - Platoon of Vehicles
 - By the Time Headway
 - Two Sites: Hartford and McLean

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37

Testing Speed Reduction

- Using Single Vehicles Data
 - Between Approach Zones: Zone A, B, and C
 - Hartford: (Oct. 1996 Group A & B), (Jul. 1997 Group G & H)
 - McLean: (Oct. 1996 Group C), (Jul. 1997 Group I and J)
- Assumption
 - H_0 : Two sets of speed are equal
 - Zone A and B ($H_0: \mu_A - \mu_B = 0$)
 - Zone B and C ($H_0: \mu_B - \mu_C = 0$)
 - Standard deviation of the difference of the means

$$\delta_{AB} = \sqrt{\frac{s_A^2}{n_A} + \frac{s_B^2}{n_B}}$$

$$\delta_{BC} = \sqrt{\frac{s_B^2}{n_B} + \frac{s_C^2}{n_C}}$$

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38

Testing Speed Reduction

- Group A:
 - Test between Zone A and Zone B
 - $\mu_A - \mu_B = 3.16 \text{ m/s} > 2 * 0.44$: at $\alpha = 0.95$
 - Reject H_0 and "Significant difference between A & B"
 - Definite tendency to reduce speed from Zone A to Zone B
 - Test between Zone B and Zone C
 - $\mu_B - \mu_C = 3.70 \text{ m/s} > 2 * 0.40$: at $\alpha = 0.95$
 - Reject H_0 and "Significant difference between B & C"
 - Definite tendency to reduce speed from Zone B to Zone C

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39

Test Results: Hartford

	Zone A & B	Zone B & C
Group A (AM, Oct. 1996)		
Mean Difference (m/s)	3.16	3.70
Std. Dev. Difference (m/s)	0.44	0.45
95 % Interval (m/s)	0.88	0.90
Speed Variation	YES	YES
Group B (PM, Oct. 1996)		
Mean Difference (m/s)	1.79	3.20
Std. Dev. Difference (m/s)	0.44	0.38
95 % Interval (m/s)	0.88	0.76
Speed Variation	YES	YES
Group G (AM, Jul. 1997)		
Mean Difference (m/s)	3.33	3.31
Std. Dev. Difference (m/s)	0.42	0.41
95 % Interval (m/s)	0.84	0.82
Speed Variation	YES	YES
Group H (PM, Jul. 1997)		
Mean Difference (m/s)	2.92	3.53
Std. Dev. Difference (m/s)	0.39	0.39
95 % Interval (m/s)	0.78	0.78
Speed Variation	YES	YES

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40

Speed Profiles for Single Vehicle

- Microscopic Speed Characteristics in Each Zone
 - Normal Distribution for the Speed Profiles
 - Zone A, B, and C
 - Utilized as input distribution for simulation
 - Chi-square Test
 - Hypothesis: the measured distribution is identical to the normal distribution
 - Estimation of size of the class interval by Sturges (1926)

$$I = \frac{\text{Range}}{\dots}$$

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41

Deceleration Rate: Single Veh.

- Based on Speed Reduction

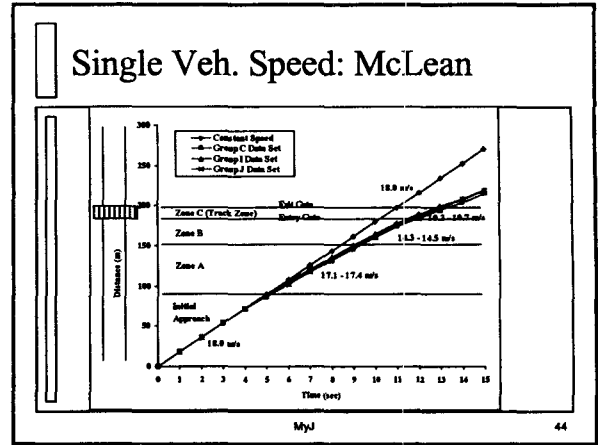
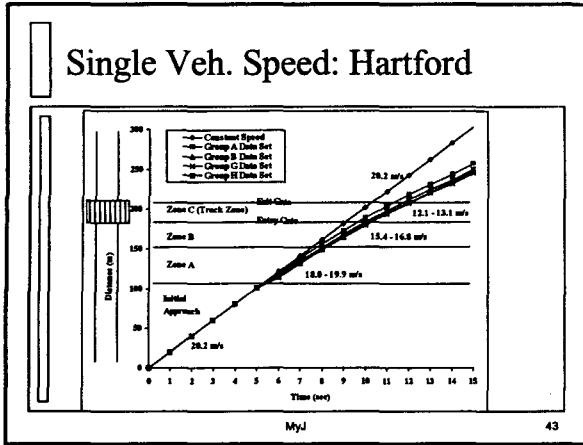
$$[a] = \frac{v^A - v^B}{t}$$

- Chi-square Test: Lognormal Distribution

Data Set	Deceleration Rates (m/s ²) Between Zone A and Zone B		Deceleration Rates (m/s ²) Between Zone B and Zone C	
	Mean	Std. Dev.	Mean	Std. Dev.
Hartford				
Group A	0.76	0.27	0.95	0.40
Group B	0.46	0.55	0.83	0.53
Group G	0.68	0.36	0.76	0.45
Group H	0.60	0.32	0.88	0.48
McLean				
Group C	0.52	0.32	1.16	0.59
Group I	0.55	0.30	1.26	0.61
Group J	0.45	0.20	1.16	0.49

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42



Platoon Vs. Single Vehicles

- Testing Speed Difference between Platoon and Single Vehicles
 - Hartford: Group (A vs. D), (B vs. E), (G vs. K), (H vs. L)
 - McLean: Group (C vs. F), (I vs. M), (J vs. N)
- Assumption
 - Two sets of speed are equal
 - for Group (A vs. D)
 - Zone A: ($H_0: \mu_{K(A)} - \mu_{L(D)} = 0$)
 - Zone B: ($H_0: \mu_{B(A)} - \mu_{E(D)} = 0$)
 - Zone C: ($H_0: \mu_{C(A)} - \mu_{F(D)} = 0$)

$$\hat{s}_{K(D)} = \sqrt{\frac{s_{K(D)}^2 + s_{L(D)}^2}{n_{K(D)} + n_{L(D)}}$$

$$\hat{s}_{B(E)} = \sqrt{\frac{s_{B(E)}^2 + s_{E(E)}^2}{n_{B(E)} + n_{E(E)}}$$

$$\hat{s}_{C(F)} = \sqrt{\frac{s_{C(F)}^2 + s_{F(F)}^2}{n_{C(F)} + n_{F(F)}}$$

MyJ 45

Test Results: Hartford

	Zone A	Zone B	Zone C
Group D&A (AM, Oct. 1996)			
Mean Difference (m/s)	0.95	1.32	1.05
Std. Dev. Difference (m/s)	0.46	0.52	0.46
95 % Interval (m/s)	0.92	1.04	0.92
Speed Difference	YES	YES	YES
Group E&B (PM, Oct. 1996)			
Mean Difference (m/s)	1.06	1.52	1.55
Std. Dev. Difference (m/s)	0.40	0.35	0.32
95 % Interval (m/s)	0.80	0.70	0.64
Speed Difference	YES	YES	YES
Group K&G (AM, Jul. 1997)			
Mean Difference (m/s)	2.28	1.80	1.42
Std. Dev. Difference (m/s)	0.36	0.39	0.36
95 % Interval (m/s)	0.72	0.78	0.72
Speed Difference	YES	YES	YES
Group L&H (PM, Jul. 1997)			
Mean Difference (m/s)	1.52	1.64	1.22
Std. Dev. Difference (m/s)	0.34	0.38	0.35
95 % Interval (m/s)	0.68	0.76	0.70
Speed Difference	YES	YES	YES

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Speed Profiles for Platoons

- Platoon Size for Hartford and McLean

Site	Data Group (No. of platoons)	2 vehicles	3 vehicles	4 vehicles	5 vehicles	more than 5 vehicles
Hartford	Group D (31)	22	8	0	1	0
	Group E (86)	52	15	7	6	6
	Group K (64)	39	10	11	4	0
	Group L (60)	45	6	4	1	4
McLean	Group F (21)	16	4	1	0	0
	Group M (13)	7	6	0	0	0
	Group N (49)	42	6	1	0	0

- Headway Characteristics: Hartford
 - Chi-square Test: Normal Distribution

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Platoon Speed Profiles: Hartford

	Zone A	Zone B	Zone C
Group D (AM, Oct. 1996): 31 Platoons	(mean, std. dev)	(mean, std. dev)	(mean, std. dev)
1 st vehicle (31 veh.), m/s	(19.39, 2.92)	(16.15, 3.12)	(12.51, 3.31)
2 nd vehicle (31 veh.), m/s	(18.81, 2.95)	(15.23, 3.12)	(12.11, 2.77)
3 rd vehicle (9 veh.), m/s	(17.94, 2.98)	(13.90, 3.13)	(10.35, 2.06)
4 th vehicle (1 veh.), m/s	(20.78, 0.00)	(16.04, 0.90)	(11.43, 0.00)
5 th vehicle (1 veh.), m/s	(22.96, 0.00)	(17.58, 0.90)	(13.40, 0.00)
Group E (PM, Oct. 1996): 86 Platoons	(mean, std. dev)	(mean, std. dev)	(mean, std. dev)
1 st vehicle (86 veh.), m/s	(18.33, 3.25)	(15.83, 3.39)	(12.45, 3.28)
2 nd vehicle (86 veh.), m/s	(16.86, 3.34)	(14.82, 3.44)	(11.59, 3.03)
3 rd vehicle (34 veh.), m/s	(16.19, 2.72)	(13.87, 3.13)	(10.89, 2.68)
4 th vehicle (19 veh.), m/s	(15.78, 2.13)	(13.75, 2.71)	(10.43, 2.60)
5 th vehicle (12 veh.), m/s	(15.46, 1.70)	(13.25, 2.95)	(10.02, 2.10)
Group K (AM, Jul. 1997): 64 Platoons	(mean, std. dev)	(mean, std. dev)	(mean, std. dev)
1 st vehicle (64 veh.), m/s	(17.00, 2.60)	(14.52, 2.70)	(11.37, 3.05)
2 nd vehicle (64 veh.), m/s	(16.85, 2.79)	(13.92, 3.12)	(10.98, 2.98)
3 rd vehicle (25 veh.), m/s	(15.44, 3.36)	(11.87, 3.74)	(9.34, 2.75)
4 th vehicle (15 veh.), m/s	(14.89, 2.72)	(11.91, 3.10)	(9.43, 2.66)
5 th vehicle (4 veh.), m/s	(15.36, 4.65)	(12.90, 4.38)	(9.65, 3.71)
Group L (PM, Jul. 1997): 60 Platoons	(mean, std. dev)	(mean, std. dev)	(mean, std. dev)
1 st vehicle (60 veh.), m/s	(18.16, 2.91)	(15.11, 3.28)	(11.80, 3.13)
2 nd vehicle (60 veh.), m/s	(17.40, 2.73)	(14.28, 3.36)	(11.29, 2.90)
3 rd vehicle (15 veh.), m/s	(16.13, 2.71)	(13.15, 3.41)	(10.71, 3.12)
4 th vehicle (9 veh.), m/s	(16.11, 2.83)	(13.66, 3.49)	(10.66, 2.56)
5 th vehicle (5 veh.), m/s	(17.54, 0.29)	(14.64, 2.14)	(10.98, 0.67)

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Summary of Speed Data Analysis

- **Speed Variation (Reduction)**
 - Using single vehicles and platoons for two sites
 - Definite tendency to reduce speed
- **Comparison bet. Platoon Vs. Single Veh.**
 - Platoon speeds are less than single vehicles'
 - In platoon, average speed of following vehicles are less than that of the leading vehicles
- **Input for Simulation**
 - Speed profiles, decel. rates, and headway

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49

DDZ Estimation for Single Veh.

- **Assumptions**
 - a train may approach at any time, the interaction between a vehicle and a train can occur
 - a single "average vehicle approaches the crossing at $t = 0$ sec
 - interacts at and within the stopping distance area with a train
- **Stopping Distance**

	At PRT = 1 sec From AASHTO (1990)	At PRT = 1.8 sec From Taoka (1989) (85 th percentile)	At PRT = 2.5 sec From AASHTO (1990)
Stopping Distance (m)	77.75	92.63	105.65
Gate Delay Needs (sec)	4.2 (-77.75/18.60)	5.0 (-92.63/18.60)	5.7 (-105.65/18.60)
Variables Used			
Approach Speed	18.60 m/s (61.02 ft/s)	18.60 m/s (61.02 ft/s)	18.60 m/s (61.02 ft/s)
Typical Decel. Rate	3.05 m/s ² (10 ft/s ²)	3.05 m/s ² (10 ft/s ²)	3.05 m/s ² (10 ft/s ²)
Grade of Crossing	0%	0%	0%

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50

DDZ Estimation for Single Veh.

- **DDZ calculation without speed reduction**
 - Hartford: by Coleman&Moon methodology
 - DDZ is minimized: $T_D = 5.70$ sec, $T_I = 2.50$ sec

Time (sec)	Accel (m/s ²)	Speed (m/s)	Vehicle Posit.(m)	Stopping Dist. (m)	Continuat. Dist. (m)	Remaining Dist. (m)	DDZ (m)
0.00	0.00	18.60	0.00	105.65	106.02	105.65	0.37
1.00	0.00	18.60	18.60	105.65	87.42	87.05	0.37
2.00	0.00	18.60	37.20	105.65	68.82	68.45	0.37
3.00	0.00	18.60	55.80	105.65	50.22	49.85	0.37
4.00	0.00	18.60	74.40	105.65	31.62	31.25	0.37
5.00	0.00	18.60	93.00	105.65	13.02	12.65	0.37
5.60	0.00	18.60	104.16	105.65	1.86	1.49	0.37
5.70	0.00	18.60	106.02				
7.00	0.00	18.60	130.20				
8.20	0.00	18.60	152.52				

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51

DDZ Estimation for Single Veh.

- **DDZ with Speed Reduction (Coleman&Moon)**

Time (sec)	Accel. (m/s ²)	Speed (m/s)	Vehicle Posit.(m)	Stopping Dist. (m)	Continuat. Dist. (m)	Remaining Dist. (m)	DDZ (m)
0.00	-0.55	18.60	0.00	105.65	97.22	105.65	8.44
1.00	-0.55	18.05	18.30	100.97	78.87	87.36	8.49
2.00	-0.55	17.50	36.05	96.39	61.06	69.61	8.55
3.00	-0.55	16.95	53.24	91.91	43.81	52.41	8.60
4.00	-0.55	16.40	69.89	87.53	27.11	35.76	8.66
5.00	-0.85	15.85	85.99	83.25	10.96	19.67	8.71
5.60	-0.85	15.42	95.36	79.93	1.54	10.30	8.75
5.70	-0.85	15.33	96.89				
7.00	-0.85	14.23	116.05				
9.70	-0.85	11.93	151.24				

- Gate Delay = 5.70 sec.
- Gate Interval Time = 4.00 sec
- DDZ is **not** minimized

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52

DDZ Estimation for Single Veh

- **DDZ with Speed Reduction**

Time (sec)	Accel (m/s ²)	Speed (m/s)	Vehicle Posit.(m)	Stopping Dist. (m)	Continuat. Dist. (m)	Remaining Dist. (m)	DDZ (m)
0.00	-0.55	18.60	0.00	105.65	106.29	105.65	0.64
1.00	-0.55	18.05	18.30	100.97	87.94	87.36	0.58
2.00	-0.55	17.50	36.05	96.39	70.13	69.61	0.52
3.00	-0.55	16.95	53.24	91.91	52.88	52.41	0.47
4.00	-0.55	16.40	69.89	87.53	36.18	35.76	0.42
5.00	-0.85	15.85	85.99	83.25	20.03	19.67	0.36
6.20	-0.85	14.91	104.43	78.12	1.49	1.22	0.27
6.30	-0.85	14.82	106.91				
7.00	-0.85	14.23	116.05				
9.70	-0.85	11.93	151.24				

- Gate Delay = 6.30 sec.
- Gate Interval Time = 3.40 sec
- DDZ is minimized

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53

Car-Following DDZ Estimation

- **Assumption**

- Last veh. of a platoon is located at the stopping distance zone when a train is approaching
- Two vehicles in a platoon
 - LV = the lead vehicle
 - TV = the target vehicle (i.e. the last vehicle)
- Three vehicles in a platoon
 - LV = the lead vehicle
 - FL = the following vehicle (i.e. the intermediate vehicle)
 - TV = the target vehicle (i.e. the last vehicle)

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54

Validation

Lead veh. speed profile: no significant difference

Lead Veh.	Speed Zone A	Speed Zone B	Speed Zone C
Average Field Speed (mph) (n=45)	18.33	15.83	12.43
Simulation Speed (mph)			
15 replications			
No. 1	18.54	17.74	15.97
No. 2	18.17	17.01	15.18
No. 3	17.29	15.23	13.05
No. 4	15.93	12.86	7.97
No. 5	24.01	22.22	18.93
No. 6	16.24	13.92	10.23
No. 7	20.24	18.90	17.04
No. 8	18.06	16.98	15.28
No. 9	20.39	18.27	16.83
No. 10	22.67	21.43	20.79
No. 11	15.27	12.20	7.90
No. 12	16.25	13.75	8.66
No. 13	15.48	13.74	11.36
No. 14	16.97	12.90	7.40
No. 15	12.23	10.29	7.47
Average	17.84	15.92	13.91
Std. Dev.	3.01	3.51	4.61
Difference	0.47	-0.09	-0.48
t-value	0.3552	0.0028	0.3711
Focus(t)	2.1448	2.1648	2.1444

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61

Validation

Test speed profiles in a platoon

- No Significant Difference
 - 1st, 2nd, 3rd, 4th, and 5th vehicles for Hartford
 - 1st, 2nd, and 3rd for McLean
- Simulation Model is Valid
 - Represent the speed profiles of vehicles in the dynamic dilemma zone methodology including car-following logic

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62

Optimal Gate Operation Times

- Findings for the optimal gate operation times
 - Using all data groups in the valid simulation model

Hartford Site

Data Group by Time and Date	Gate Delay (sec)	Gate Interval Time (sec)
Group DAA (AM, Oct. 1996)	6.60	6.20
Group EAB (PM, Oct. 1996)	6.10	6.20
Group KAC (AM, Jul. 1997)	6.30	6.30
Group LAH (PM, Jul. 1997)	6.30	6.30

McLean Site

Data Group by Time and Date	Gate Delay (sec)	Gate Interval Time (sec)
Group FBC (AM, Oct. 1996)	6.10	4.90
Group MBI (AM, Jul. 1997)	6.10	4.90
Group MAJ (PM, Jul. 1997)	6.00	4.90

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63

5 Conclusions and Recommendations

Review of Objectives

- Dynamic Dilemma Zone and Car-Following
- Optimal Gate Delay and Gate Interval Time
- Statistical Distribution of Driver Behavior

Summary and Conclusions

- Statistical Distribution of Driver Behavior
 - Definite tendency of speed reduction
 - Average speed of platoon is less than that of single vehicle
- DDZ and Car-Following Model
 - DDZ exists in both cases of constant speed and speed reduction including car-following logic

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64

Conclusions and Recommendations

Summary and Conclusions (cont'd)

- Simulation Model for Optimal Gate Operation Times
 - Gate delay and gate interval time based on minimizing DDZ
- Validation
 - The proposed DDZ and Car-Following model is valid by comparing speed profiles with field data
- Optimal Gate Operation Times are Obtained
 - Using all data groups for both sites: Hartford and McLean

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65

Conclusions and Recommendations

Recommendations

- More data collection for interaction between veh. & train
- Test and refine model for multi-lane HRIs
- Applications
 - Evaluating intergreen interval, yellow and all-red at HHI
 - Intelligent Transportation Systems (ITS)
 - Real-time traffic control at HHI, HRI, work-zone, incident area, merging area, lane closure, toll collection area, etc.
 - Advanced Traffic Management System (ATMS)
 - Automated Highway System (AHS)

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66